Rheological Behavior of Recycled Fiber Reinforced Self-Compacted Concrete with Recycled Coarse Aggregate

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Abstract: It is known that self-compacted concrete SCC is used to serve the construction of concrete structures with complicated shapes having congested reinforcements. Sustainable SCC is produced now by recycled concrete aggregates RCA from construction wastes. To improve the strength of this concrete, with a further eye on sustainability, recycled steel fibers RSF is utilized as reinforcing to the SCC. In this work, 12 SCC mixes were prepared with different mixing proportions. Cement is replaced with 5% silica fume. RCA replacement percentages were 25%, 50%, and 100% RCA. RSF was used with 0.50%, 0.75% by volume of concrete. The slump flow, J-Ring, and V-Funnel tests are performed to estimate the rheological behavior of all concrete mixtures. Results indicate that RCA/RSF SCC passed the slump flow, J-Ring tests, and overpassed V-Funnel test requirements satisfactorily.

Keywords: Self-Compacting Concrete, Recycled Aggregate (RCA), Recycled Steel Fiber (RSF), Silica Fume, And Fresh Properties

1. Introduction

The constructions of substantial concrete structures have been increased with designed complex forms and complicated reinforcements that make the workability of the fresh concrete to be unworkable and efficiently burdensome. Japanese scholars developed a new kind of concrete during 1980th, which is known as self-compacting concrete (SCC). SCC is one of the particular types which without mechanical vibrating, attains compaction. During its plastic state flowing under its weight and protects, it is homogeneousness throughout filling every form like the framework's congested reinforcement (Revathi, Selvi, & Velin, 2013). A massive quantity of construction wastes produced annually from demolished buildings, earthquakes, natural disasters, construction places, materials of construction, and factories of materials (Al-Numan, 2019/12; Revathi et al., 2013; Younis & Latif; Younis & Pilakoutas, 2013).

Because of inadequate removal sites, especially around big cities for eliminating and removing the wastes, a significant problem occurs. In contrast, utilizing natural concrete aggregate is increased too much due to urging construction activity owing to the fast growth of urbanization. Similarly, to overcome this problem in the construction industry, constructing sustainable concrete needs to be considered. So the researchers practiced a sustainable technique to achieve an innovative concrete kind by using the demolished waste of construction efficiently. Recycling and utilizing the concrete wastes is very significant in nature protection since it can reduce the environment pollution and protects nature by decreasing the usage of natural aggregate resources (Omrane, Kenai, Kadri, & Aït-Mokhtar, 2017). However the fresh property of the concrete is affected negatively by utilizing recycled concrete aggregate since the slump loss of RCA is more than the NCA (Abed & Nemes, 2019; CarroYLópez et

al., 2017; Debieb, Courard, Kenai, & Degeimbre, 2009; Hama & Hilal, 2017); generally, this is because of the shape, size, water content and texture of the RCA that influenced profoundly on the fresh property of the concrete (Abed & Nemes, 2019; Itsubo, Sakagami, Washida, Kokubu, & Inaba, 2004; Purushothaman & Mani, 2014; Revathi et al., 2013).

On the other hand, in order to improve the strength of the concrete, scholars try to use many types of fibers for commercial applications and reinforcement of concrete; usually, some of them are existing. Steel, polypropylene, natural cellulose, nylon, carbon, and glass are some kinds of fibers (Mermerdaş, Mulapeer, & Oleiwi, 2019). Several fiber types are available to be used in concrete reinforcement. Besides their sources, they have different mechanical properties, density, geometry, and shape. Apart from steel fiber, glass and polypropylene are the most used fiber types. Steel is the most used structural fibers in the construction industry. They have different shapes, performance levels, and geometries. There most common steel fiber geometries are straight, hooked, deformed, and coned end.

The attention of the researchers attracted to use recycled steel fiber instead of fabricated steel fiber as a reinforcement matrix in cement base materials over the last decade, regarding the high cost of steel fiber, and it is friendly to the environment (Mastali & Dalvand, 2016). From the results of the fresh tests, usually, the passing ability and flowability of the fresh concrete are significantly reduced by adding RSF of scraped tires. Generally, the fresh properties of concrete reduced gradually by using RSF. The length and amount of RSF affect the fresh property of the SCC (Younis, Ahmed, & Najim, 2018). By increasing the amount of RSF, the flowability is decreased, as shown from the results of Simalti and Singh (2019) and a similar outcome attained by (Groli, Pérez Caldentey, & Soto, 2014; Mastali & Dalvand, 2016, 2017). The workability and homogeneity of the fresh concrete are inversely proportional to the amount of RSF(Liew & Akbar, 2020). This study investigates the influence of utilizing both RCA and RSF on fresh properties of SCC, like flowability, passing ability, and viscosity of the fresh concrete by Slump Flow, J-Ring, and V-Funnel test.

2. Experimental Details

2.1 Materials

The materials used in this research are (cement, silica fume (SF), natural concrete aggregate (NCA), recycled coarse aggregate (RCA), natural fine aggregate (NFA), recycled steel fiber (RSF), water and superplasticizer. Ordinary Portland cement (OPC) type CEM I 42.5 R is used. Silica fume is also used by replacement of 5% of cement. The physical properties and the chemical compositions of the cement and the silica fume are included in Tables 1 and 2, respectively.

Chemical compositions %	CEM I 42.5 R
CaO	67.46
SiO ₂	13.48
Al ₂ O ₃	3.69
Fe ₂ O ₃	7.78
MgO	1.29
Na ₂ O	0.36
K ₂ O	0.98
SO ₃	4.82
Physical Properties	CEM I 42.5 R
Specific Weight (g/m3)	3.18
Specific surface area (Blaine (cm2/g)	3352
Loss on ignition	1.98

Table 1: Chemical composition and physical properties of Cement

Table 2: Chemical composition and physical properties of Silica Fume

Chemical compositions %	Silica Fume
CaO	1.6
SiO ₂	92.3
Al ₂ O ₃	1.3
Fe ₂ O ₃	1
MgO	0.9
Na ₂ O	0.25
K ₂ O	0.79
SO ₃	0.11
Specific Weight (g/m ³)	2.2
Specific surface area (Blaine (cm ² /g)	21
Loss on ignition	1.53

River natural fine aggregate and coarse aggregate of having a specific gravity of 2.67 and 2.65, respectively, are used. The natural coarse aggregate is replaced by recycled coarse aggregate with 25%, 50%, and 100%. The RCA is obtained from a demolished reinforced concrete building in Erbil. The samples were taken from a slab of the demolished building; after that, the slab is broken for smaller pieces, not more than 25 mm in size. The RCA is sieved with (No. 5/8), the passed RCA is used. The Chemical composition and physical properties of NCA, RCA, and NFA are listed in Tables 3 and 4, respectively.

Table 3: Physical properties of Aggregates

Physical Properties	CA	NA	RCA
Fines Modulus	6.6	2.55	6
Specific gravity (g/m ³)	2.64	2.52	2.5
Water absorption (%)	0.4	0.25	3

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The RSF is obtained from consumed old tires; by pyrolysis process the steel fiber has been extracted out from the tire. The physical properties of the RSF are listed in Table 4.

Physical Properties	RSF
Diameter (mm)	0.3
Length (mm)	10.0-20.0
Specific gravity	7.8

Table 4: Physical properties of recycled steel fibers

For increasing the workability, high-performance concrete superplasticizer (Formerly known as Flocrete PC200) is used. The chemical and physical properties are listed in Table 5.

Table 5: Chemical compositions and physical properties of superplasticizer

Chemical compositions and Physical Properties	Superplasticizer
Form	Liquid
Color	Light Yellow
Odor	Slight/Faint
Boiling Point (C)	> 100
Flash Point (closed up) (C)	None
Autoflamibilty	None
Explosive Properties %	None
Freezing point	-3
Relative Density	1.03- 1.07
Water Solubility	Soluble

2.2 Mixture Proportions

Twelve mixes were prepared with different mixing proportions. The first mix is a control mix without RCA and RSF, and the second one is 25% RCA, the third one with 50% RCA, and the fourth one with 100% RCA. From the 5th through the 8th mix, (0.5 %) RSF was used constantly with 25%, 50%, and 100% RCA, and from the 9th mix through the 12th (0.75%) RSF constantly used with 25%, 50% and 100% RCA. Table 6 includes details of all mixes.

Table 6: Mixture pr	coportions (kg/m^3)
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Mixes	M1	M2	M3	M4	M5	M6	M7	M8	M9	M10	M11	M12
С	475	475	475	475	475	475	475	475	475	475	475	475
SF	25	25	25	25	25	25	25	25	25	25	25	25
W	150	150	150	150	150	150	150	150	150	150	150	150
SP	15	15	15	15	15	15	15	15	15	15	15	15
RSF	0	0	0	0	39	39	39	39	59	59	59	59
NCA	810	607	405	0	804	603	402	0	801	600	400	0
RCA	0	196	391	782	0	194	388	777	0	193	387	774
FA	982	982	982	982	975	975	975	975	971	971	971	971

C: Cement, SF: silica Fume, W: Water, SP: superplasticizer, RSF: recycled steel Fiber, NCA: normal concrete Aggregate, RCA: recycled concrete aggregate, FA: fine aggregate.

2.3 Specimens Casting and Curing

One hundred forty-two samples were prepared for twelve different mixes. From each mix, six cubes with size (100x100x100) mm and six prisms with size (100x100x500) mm were prepared. Fresh properties tests such as Slump Flow, J-Ring, and V-Funnel were performed for all the mixes.

2.4 Test Methods

2.4.1 Slump Flow, J-Ring and V-Funnel

Slump flow is used to evaluate the flowability and deformability of fresh properties of all the mixes of concrete. ASTM C 1611 is used to achieve the slump flow. For designing the self-compacting concrete mixes, the acceptance rate is between (500 to 750) mm, as stated by BIBM and ERMCO (2005). The passing ability of the self-compacting concrete through obstacles is measured by using the J-Ring test method. ASTM (C1621/C 1621M-09) is used to perform the test, as mentioned in BIBM and ERMCO (2005). The filling ability and flowability of the fresh properties of Self-compacting concrete are measured by using the V-funnel test. The test is achieved by measuring the period of passing fresh self-compacting concrete through an opening that narrowed at the end of the v-funnel. The period required for the test is between (8-25) seconds, as described by EFNARC (BIBM & ERMCO, 2005). The test apparatus of Slump flow, J-Ring, and V- funnel is shown in Figure 1.



Figure 1: The test apparatus

3. Results and Discussion

Table 7 shows all the results of fresh properties.

Mixes	Slump Flow	J-Ring	V-Funnel
	(mm)	(mm)	(Second)
M1(NCAScc)	725	715	9
M2(RCA1Scc)	705	685	11
M3(RCA2Scc)	665	655	11.5
M4(RCA3Scc)	655	650	12
M5(RSF0.5NCAScc)	680	655	11
M6(RSF0.5RCA1Scc)	675	650	17
M7(RSF0.5RCA2Scc)	655	575	32
M8(RSF0.5RCA3Scc)	645	555	38
M9(RSF0.75NCAScc)	675	650	11.5
M10(RSF0.75RCA1Scc)	650	575	24
M11(RSF0.75RCA2Scc)	635	570	34
M12(RSF0.75RCA3Scc)	620	510	40

Table 7: Test results for various SCC mixes

Scc: Self-compacting concrete, NCA: Natural concrete aggregate, RCA: Recycled concrete aggregate: 1 for 25%, 2 for 50%, 3 for 100% replacement, RSF: recycled steel fiber: 1 for 0.5%, 2 for 0.75% by volume.

3.1 Slump Flow Results

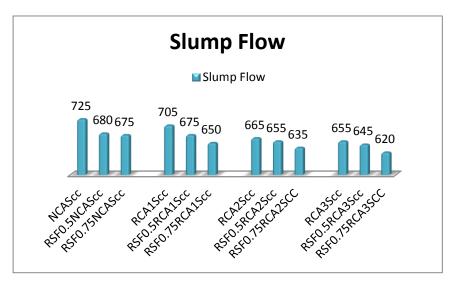
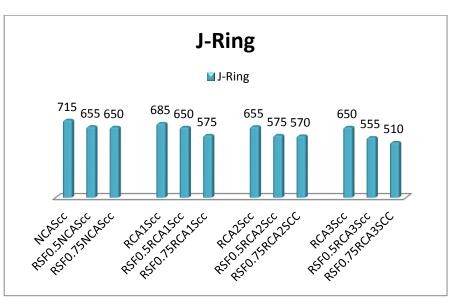


Figure 2: Slump flow results (mm)

As shown in figure 2, the slump flows of all mixes satisfy the SCC requirements. Although variation is indicated as shown in the results, the flowability of the control mix is more than rest mixes. As it is

seen from the results, the slump flow for the first mix in which no RCA and RSF used is 725 mm. As the RCA replacement and/or RSF percentages are increased, the slump flow is reduced. The last mix with 0.75% RSF and 100% RCA used slump flow reduced to 620 mm; however, all the mixes are within the acceptable range. This was confirmed by (Omrane et al., 2017; Safiuddin, Salam, & Jumaat, 2011; Silva, Robayo, Mattey, & Delvasto, 2016) for RCA replacement. It can be explained that the angular shape and rough surface of the RCA opposite to the NCA are affecting the behavior (Abed & Nemes, 2019; Mathew, 2015). Also, the water absorption of RCA is more than that of NCA, which can be considered as an effective factor.

It can be added that when introducing RSF to the SCC/RCA mixes, the slump flow is reduced further. (Younis et al., 2018). For instant, in the fifth and the ninth mixes in which fully NCA is used with 0.5 % and 0.75% of RSF, the slump flow became 680 mm and 675 mm, respectively, which are less than the first mix. For the last three mixes in which 0.75% RSF is used with 25%, 50%, and 100% RCA, the flowability decreased to 650mm,635mm, and 620 mm, respectively.



3.2 J-Ring Results

Figure 3: J-Ring results (mm)

Figure 3 shows that the passing ability from J-Ring tests of all mixes satisfies the SCC requirements. The control mix shows the greatest passing ability over the rest of the mixes. By increasing the percentage of RCA, the passing ability is reduced. This is as confirmed by Safiuddin et al. (2011). Same as the slump flow results -as it is shown in figure 3- all the mixes are in the range of SCC in which they record passing ability 715 mm to 510 mm. After replacing NCA by RCA with 25%, 50%, and 100% for 2nd, 3rd, and 4th mixes, the passing ability reduced to 685mm, 655mm, and 650 mm, respectively.

It can be added hereby that by introducing RSF to the SCC mixes, the passing ability is reduced. From the 5th through the 8th mix after adding 0.5% RSF, the passing ability decreased more due to the effectiveness of the RSF as follows 655mm, 650, 575mm, and 550mm corresponding to RCA with 0%, 25%, 50%, and 100%. Moreover, with 0.75% RSF, in the last four mixes, the less passing ability was recorded, which is a reduction from 650mm to 510 mm, see figure 3.

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3.3 V-Funnel Results

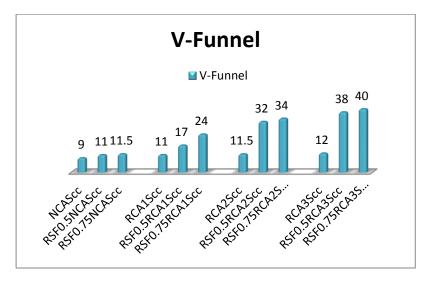


Figure 4: V-Funnel results (second)

For the V-funnel test, the first four mixes results satisfy the requirements for SCC, which is in-between 8-25 seconds, as indicated by BIBM and ERMCO (2005). In the first mix, the period for the test is 9 seconds. After Replacing NCA by RCA by 25%, 50%, and 100%, the time is increased to 11, 11.5, 12 seconds, respectively, which remained within the standard range. Adding 0.5% RSF to ordinary concrete increases further the period from 11 to 17 seconds (see fifth and sixth mixes); the time increases from 11 to 17 seconds; however, it remains within the standard time. When replacing RCA with 50% and 100%, the time is increased to 32 and 38 seconds, which passes the range and does not satisfy SCC requirements.

With 0.75 % of RSF (see 9th and 10th mixes), the time is increased from 11.5 to 24 seconds, however, still within the range. On the other hand, with the replacement of RCA to 50% and more, time reaches 34 and 40 seconds, which passes the range. It indicates that using both RCA and RSF together considerably affects flowability in which both of them together increases the viscosity and make an obstacle for the flowability. However, it can be argued that the V-Funnel test is inherently not suitable for this type of concrete, and it suffices to use slump flow and J-ring tests.

4. Conclusion

Sustainable SCC with RCA partially or fully replacing NCA and reinforced with RSF is produced in this work and tested for rheological behavior. From the results, the following conclusions can be drawn.

- 1. SCC passed the flow test, J-ring test, and the V-Funnel test. All values were within the acceptable limits.
- 2. Replacing NCA by RCA partially or fully in the SCC caused a decrease in all flow values. The entirely replaced RCA yields the lowest flow values. However, all values remain within acceptable limits.
- 3. Adding RSF to SCC decreased the flow values; the 0.75% by volume mixes yields the lowest flow result.

- 4. Utilizing both RCA and RSF shows a further decrease in inflow for all tests. However, all values remain within the acceptable limits, except the V-Funnel test result for 50% or fully replaced RCA together with the RSF.
- 5. It can be argued that the V-Funnel test is inherently not suitable for this type of concrete, and it suffices to use slump flow and J-ring tests.

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